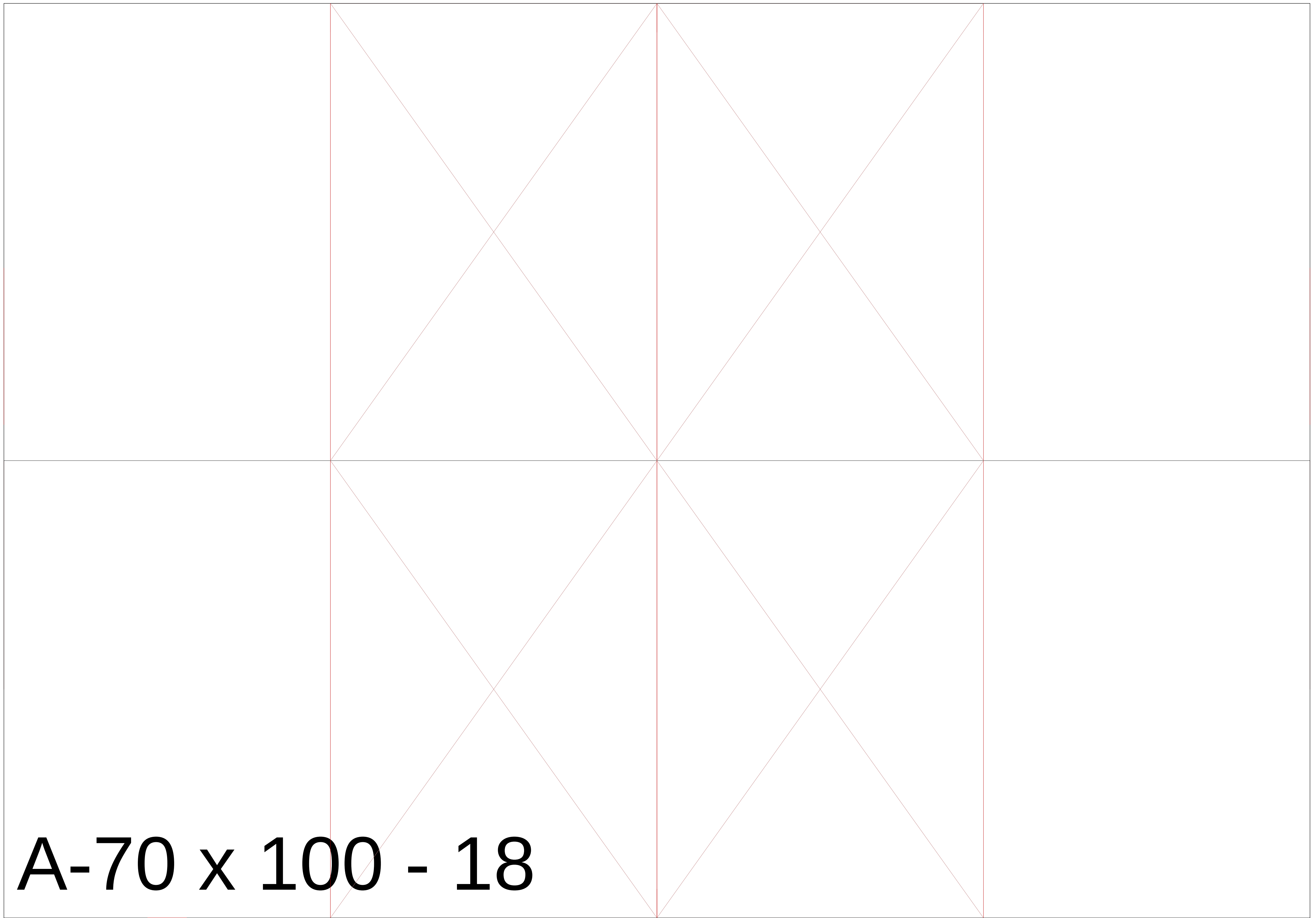
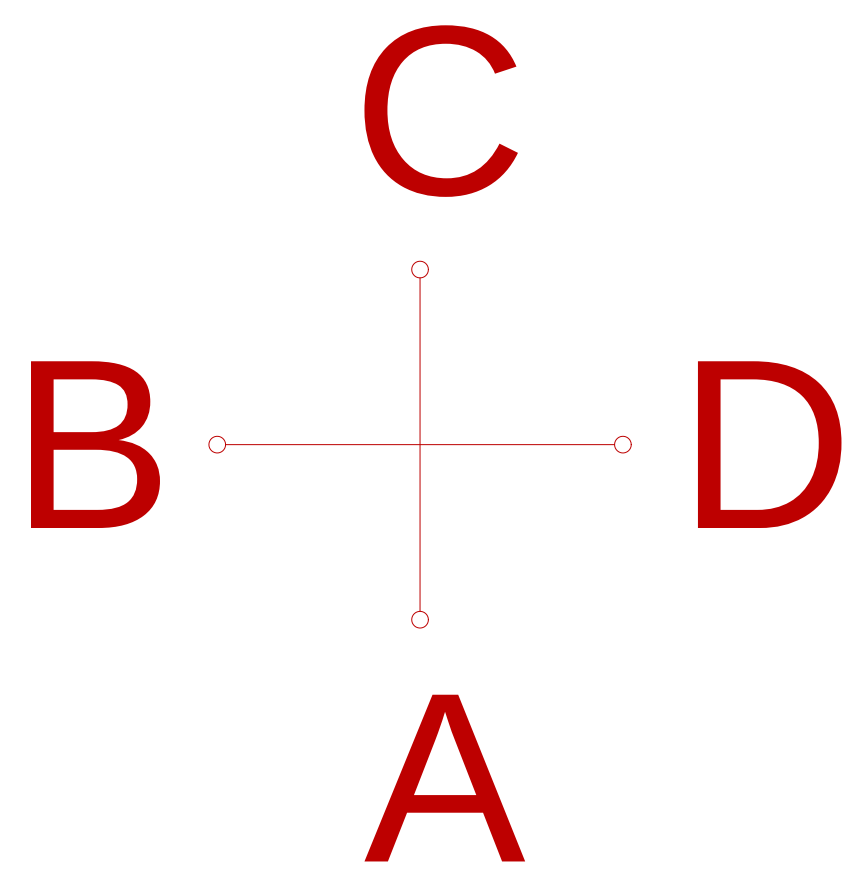




A-70 x 100 - 18

The diagram shows a rectangular area with a total width of 100' and a total height of 18'. The area is divided into four equal sections, each 25' wide. The entire area is filled with vertical lines, and the dimensions are labeled on the left and bottom.

This elevation drawing shows a building with a gambrel roof. The overall height is 18 feet. The roof has a 12-foot horizontal run for each of its two slopes, with a 2-foot 9-inch gap between the slopes at the peak. The building has a total width of 70 feet, divided into four 17-foot 6-inch sections. A central window is 14 feet high and 12 feet wide, positioned 2 feet 9 inches from the left edge of the building.

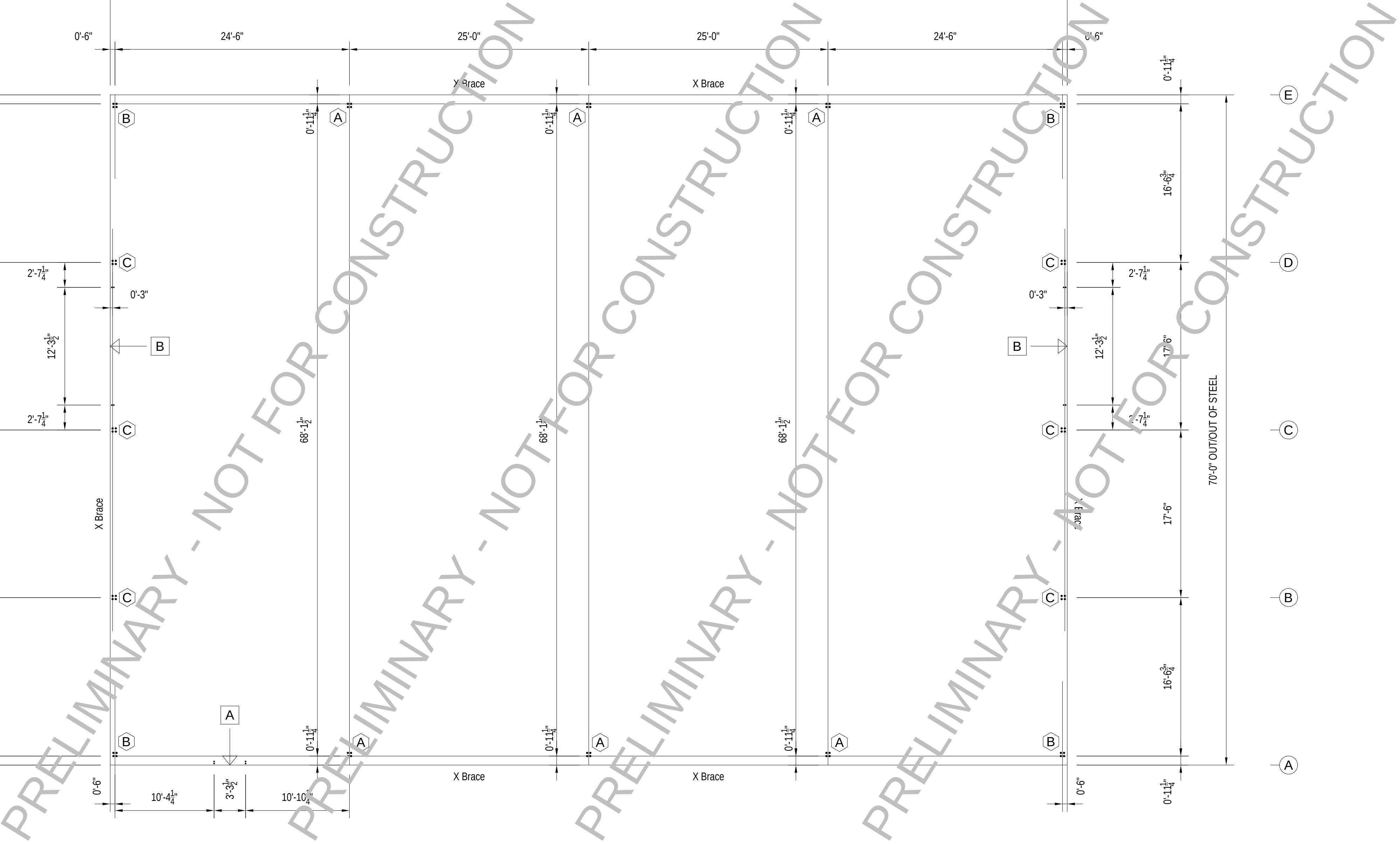
Technical drawing of a building elevation. The structure features a gabled roof and vertical siding. The overall height is 18'. The total width is 70'. The elevation includes a central door with a width of 12' and a height of 14'. The door is positioned 2'-9" from the centerline. The roof slope is indicated by a dashed line. The drawing is divided into four sections by vertical lines, with horizontal dimensions of 17'-6" for each section.

ARCHITECTURAL - (A) 70 x 100 - 18

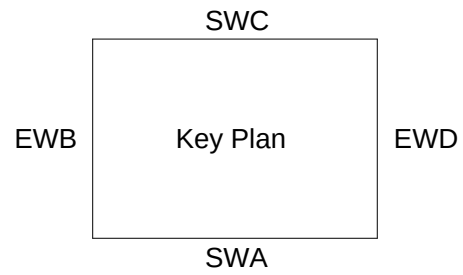
		<i>Builder:</i> TORO STEEL BUILDINGS USA		<i>Customer:</i>	
<i>Scale:</i> NOT TO SCALE		<i>Drawing Status:</i>		<i>For Construction Permit</i>	
VERSION		PAPER SIZE		☐ Preliminary	☐ For Erector Installation
ABS 2.1.9		22x34		☐ For Approval	☐
ESTIMATOR		DATE			
		11/24/025			
JOB NAME					
373251					
MEMBER					
					
The engineer whose seal appears hereon is an employee for the manufacturer for the materials described herein. Said seal or certification is limited to the products designed and manufactured by manufacturer only. The undersigned engineer is not the overall engineer of record for this project.					

- 1 This drawing is for anchor rod placement only and is not a foundation design.
- 2 Foundation must be square and square level with all anchor rods true in size, location, and projection.
- 3 Projection shown must be held to keep threads clear of finished concrete.
- 4 This structural design data includes magnitude and location of design loads and support conditions. The designer is responsible for determining the design load member numbers necessary to show compliance with the order documents at the time of this issue. Any change to building loads or dimensions may change structural member sizes and design loads. The designer is responsible for determining the design load member numbers for any future loading.
- 5 The length of anchor rods determined by shear and tension at the bottom of the base plate.
- 6 The length of anchor rod and method of load transfer to the foundation are to be determined by the foundation engineer and are not provided by the manufacturer.
- 7 Anchor Rods and nuts shall be 3000 psi concrete compressive strength (FC).
- 7 3000 psi concrete compressive strength (FC) is assumed for the purpose of column base plate design unless otherwise noted.
- 8 The anchor rods are provided for the column base plates are not shown on manufacturing jacks.
- 9 End user may or may not install concrete between frames framed anchor jacks.

The drawings herein are designed according to the indicated building code and provisions (Basis of Design) that have been supplied by the builder, project owner, or an Architect and/or Engineer of Record for the overall construction project. The Reactions and Anchor Rod Drawings are prepared based on the Basis of Design and may not represent the final project in every detail. Any deviation from the Basis of Design or modification of the plans, by the End User or Builder is done at their own risk. The information contained within these plans is intended to depict the design and basic construction details to initiate the design of the building concrete foundation.



This drawing is not for construction. This drawing is intended to depict general building information and is solely for sales presentation purposes. For clarity of presentation, items depicted may be different from actual design and final drawings. In the event of conflict between this drawing and the purchase order, the purchase order shall prevail.



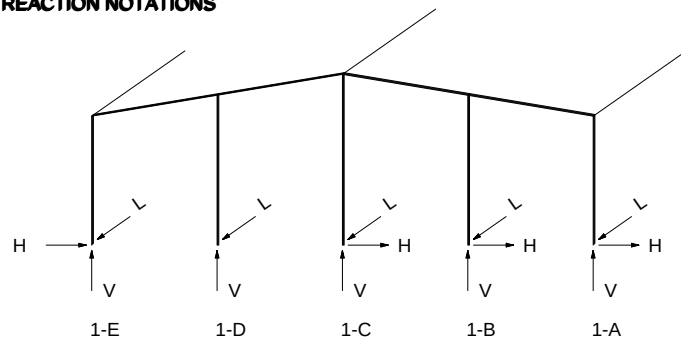
ANCHOR BOLTS TO BE DESIGNED BY FOUNDATION ENGINEER USING DIAMETERS SHOWN IN THE TABLE	
ANCHOR ROD DESCRIPTION	QUANTITY
5/8"Ø DIAMETER X	12
3/4"Ø DIAMETER X	64

ACCESSORY SCHEDULE			
Mark	Description	Details	Qty
<u>A</u>	3'-0" X 7'-0" FRAMED OPENING	<u>D</u>	1
<u>B</u>	12'-0" X 14'-0" FRAMED OPENING	<u>D</u>	2

ANCHOR ROD PLAN - (A) 70 x 100 - 18

	Revision	Date	Description	By	Ck'd
Customer: Project Name & Location: 373251 ☐ Preliminary (Not For Construction) ☐ For Approval (Not For Construction) ☐ For Construction Permit ☐ For Erector Installation					
Scale: NOT TO SCALE					
Drawn By: Auto 11/24/25					
Checked By:					
Project Engineer:					
Job Number:					
Sheet Number: F1 of 4					
The engineer whose seal appears hereon is an employee for the manufacturer for the materials described herein. Said seal or certification is limited to the products designed and manufactured by manufacturer only.The undersigned engineer is not the overall engineer of record for this project.					

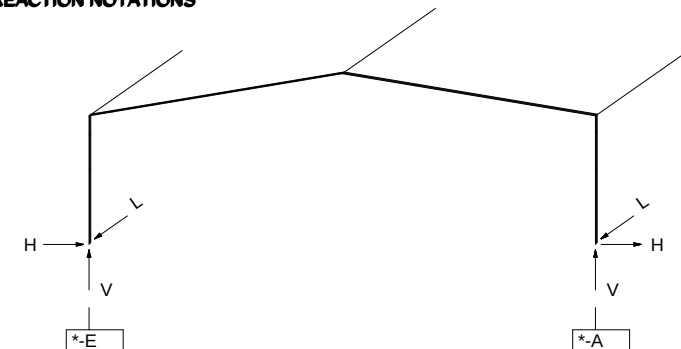
PATH: C:\Workspace\2.3.3\res\373251\		FRAME DESCRIPTION: Endwall EWB	USER NAME: JOB NAME:	DATE: 11/24/25 FILE: REW3BLDG1	PAGE: EW 1
SUPPORT REACTIONS FOR EACH LOAD GROUP NOTE: All reactions are in kips and kip-ft.					TIME: 20:30:07



LOAD GROUP REACTION TABLE															
COLUMN	1-E			1-D			1-C			1-B			1-A		
LOAD GROUP	H	V	L	H	V	L	H	V	L	H	V	L	H	V	L
D	0.0	0.5	0.0	0.0	1.1	0.0	0.0	0.8	0.0	0.1	0.0	0.5	0.0	0.0	0.0
C	0.0	0.4	0.0	0.0	1.2	0.0	0.0	0.8	0.0	0.1	0.0	0.4	0.0	0.0	0.0
L	0.0	1.8	0.0	0.0	5.3	0.0	0.0	3.3	0.0	0.5	0.0	1.8	0.0	0.0	0.0
S	0.0	2.0	0.0	0.0	6.0	0.0	0.0	3.8	0.0	0.6	0.0	2.0	0.0	0.0	0.0
SBAL	0.0	2.0	0.0	0.0	6.0	0.0	0.0	3.8	0.0	0.6	0.0	2.0	0.0	0.0	0.0
SUP	0.0	1.8	0.0	0.0	7.5	0.0	0.0	3.9	0.0	0.18	0.0	0.6	0.0	0.0	0.0
W	0.0	0.6	0.0	0.0	1.9	0.0	0.0	1.9	0.0	0.7	0.0	1.8	0.0	0.0	0.0
W+	-0.1	-2.2	1.7	0.0	-7.1	2.9	0.0	-6.5	4.3	0.0	-7.1	3.8	0.1	-2.2	1.7
W-	-0.1	-2.2	-2.1	0.0	-7.1	-4.1	0.0	-6.5	-4.7	0.0	-7.1	-4.1	0.1	-2.2	-2.1
W+	-0.1	-2.2	2.1	0.0	-7.1	4.1	0.0	-6.5	-4.3	0.0	-7.1	4.1	0.1	-2.2	2.1
WL	-2.5	-2.2	0.0	0.0	-1.1	0.0	-2.4	-9.3	0.0	0.0	-4.3	0.0	-2.8	-2.2	0.0
E+	0.0	0.0	0.0	0.0	0.1	0.0	0.0	0.1	0.0	0.0	0.1	0.0	0.0	0.0	0.0
E-	0.0	0.0	0.0	0.0	-0.1	0.0	0.0	-0.1	0.0	0.0	-0.1	0.0	0.0	0.0	0.0
ER	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
ER	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.6	0.0	0.4	-0.6	0.0	0.0	0.0	0.0

LOAD GROUP DESCRIPTION	
D	Dead load
C	Collateral load
L	Live load
S	Uniform snow load
SBAL	Balanced roof/snow
SUR	Unbalanced snow - Wind from right to left
SUL	Unbalanced snow - Wind from left to right
W+	Wind load as an inward acting pressure
W-	Wind load as an outward acting suction
WR	Wind force from the right
WL	Wind force from the left
E+	Seismic force acting inward
E-	Seismic force acting outward
EL	Seismic force from left
ER	Seismic force from right

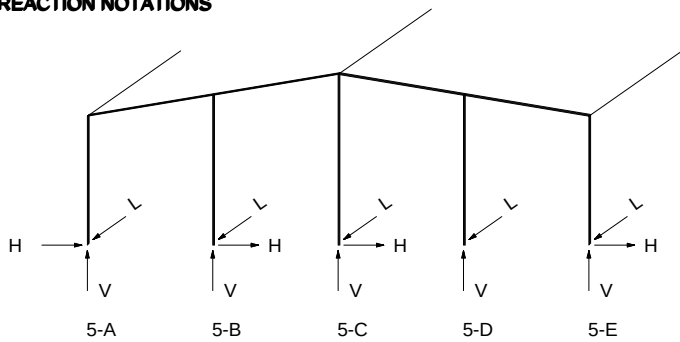
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CA	70/18/25	JOB NAME:		FILE:		Frame_3rfa	
SUPPORT REACTORS FOR EACH LOAD GROUP							
*LOCATION: Gridlines							
NOTES: (1) All reactions are in kips and kip-ft.							
(2) The seismic overstrength factor (Omega) is not included in the "RBDIEWQ" and "RBUPEQ" Load Group reactions.							
(3) Seismic "BASE-ONLY" combination reactions include an overstrength factor of: 2.500							
(4) Primary wind load cases are not concurrent.							
(5) X-bracing reactions (RBPULW and RBUPEQ) are combined with LWL and LEQ groups only.							



COLUMN	*E			*A		
LOAD GROUP	H	V	L	H	V	L
LS	10.9	19.4	-0.0	-10.9	11.3	-0.0
RBDDLW	-0.0	2.4	-0.0	0.0	2.4	-0.0

LOAD GROUP DESCRIPTION	
LS	: Unbalanced Left Roof Snow Load
RBDWLW	: Downward Acting Rod Brace Load from Long. Wind

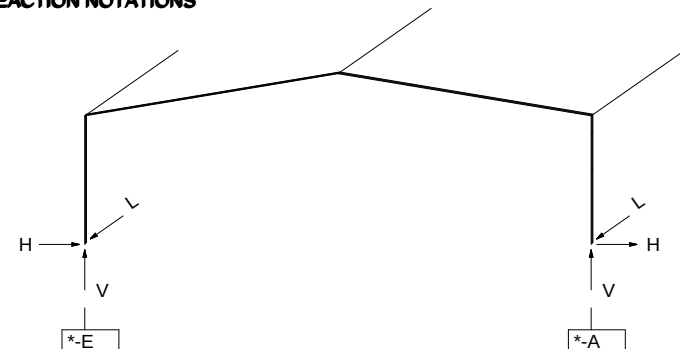
PATH: C:\Workspace\2.3\res\373251\		FRAME DESCRIPTION: Endwall EWD	USER NAME: JOB NAME:	DATE: 11/24/25 FILE: REW4BLDG1	PAGE: EW 2
SUPPORT REACTIONS FOR EACH LOAD GROUP NOTE: All reactions are in kips and kip-ft.					TIME: 20:30:07



LOAD GROUP REACTION TABLE																	
COLUMN			5-A			5-B			5-C			5-D			5-E		
LOAD GROUP	H	V	L	H	V	L	H	V	L	H	V	L	H	V	L		
D	0.0	0.5	0.0	0.0	1.1	0.0	0.0	0.8	0.0	0.0	1.1	0.0	0.0	0.5	0.0		
C	0.0	0.4	0.0	0.0	1.2	0.0	0.0	0.8	0.0	0.0	1.2	0.0	0.0	0.4	0.0		
L	0.0	1.8	0.0	0.0	5.3	0.0	0.0	3.3	0.0	0.0	5.3	0.0	0.0	1.8	0.0		
S	0.0	2.0	0.0	0.0	6.0	0.0	0.0	3.8	0.0	0.0	6.0	0.0	0.0	2.0	0.0		
SBAL	0.0	2.0	0.0	0.0	6.0	0.0	0.0	3.8	0.0	0.0	6.0	0.0	0.0	2.0	0.0		
W	0.0	1.8	0.0	0.0	7.5	0.0	0.0	3.9	0.0	0.0	1.8	0.0	0.0	0.6	0.0		
SUL	0.0	0.6	0.0	0.0	1.8	0.0	0.0	3.8	0.0	0.0	7.5	0.0	0.0	1.8	0.0		
W+	-0.1	-2.2	1.7	0.0	-7.1	-2.9	0.0	-6.5	4.3	0.0	-7.1	-3.8	0.1	-2.2	1.7		
W-	-0.1	-2.2	-2.1	0.0	-7.1	-4.1	0.0	-6.5	-4.7	0.0	-7.1	-4.1	0.1	-2.2	-2.1		
WR	-0.1	-2.2	0.0	0.0	-7.1	0.0	0.0	-6.5	2.0	0.0	-7.1	0.0	0.0	2.5	0.0		
WL	-2.5	-2.2	0.0	-2.4	-0.4	0.0	0.0	-3.3	0.0	0.0	-7.1	0.0	-2.8	-2.2	0.0		
E+	0.0	0.0	0.0	0.0	0.0	0.1	0.0	0.0	0.1	0.0	0.0	0.1	0.0	0.0	0.0		
E-	0.0	0.0	0.0	0.0	-0.1	0.0	0.0	0.0	-0.1	0.0	0.0	-0.1	0.0	0.0	0.0		
ER	0.0	0.0	0.0	0.0	-0.6	0.0	0.0	0.6	0.0	0.0	0.0	0.0	0.0	0.0	0.0		
ER	0.0	0.0	0.0	0.0	0.5	0.0	0.0	0.4	-0.5	0.0	0.0	0.0	0.0	0.0	0.0		

LOAD GROUP DESCRIPTION	
D	Dead load
C	Collateral load
L	Live load
S	Uniform snow load
SBAL	Balanced roof snow
SUR	Unbalanced snow - Wind from right to left
SUL	Unbalanced snow - Wind from left to right
W+	Wind load as an inward acting pressure
W-	Wind load as an outward acting suction
WR	Wind force from the right
WL	Wind force from the left
E+	Seismic force acting inward
E-	Seismic force acting outward
EL	Seismic force from left
ER	Seismic force from right

FRAME ID # 1	USER NAME: vmusac03prds18	DATE: 11/24/25	PAGE: 1 - 2
cs 70/18.76/25 70/135/3	JOB NAME:	FILE: frames_2_4.rta	
SUPPORT REACTIONS FOR EACH LOAD GROUP			
LOCATION: Gridlines 2 4			
NOTES: (1) All reactions are in kips and kip-ft.			
(2) The seismic overstrength factor (Omega) is not included in the "RBDWQV" and "RBRUFQV" Load Group reactions.			
Seismic "BASE-ONLY" combination reactions include an overstrength factor of: 2.500			
(3) Primary wind load cases are not concurrent.			
(4) X-bracing reactions (RBPULW and RBPULV) are combined with LWL and LEQ groups only.			
			TIME: 20:55

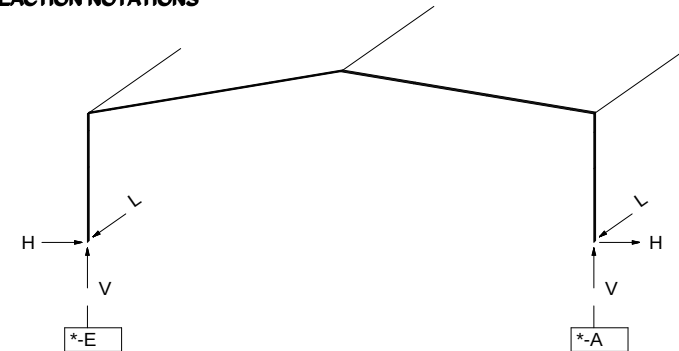


LOAD GROUP REACTION TABLE GRIDLINES * = 2 4						
COLUMN	*E			*A		
LOAD GROUP	H	V	L	H	V	L
DL	1.9	3.3	-0.0	-1.9	3.3	-0.0
COLL	2.5	3.9	-0.0	-2.5	3.9	-0.0
SNOW	12.6	19.4	-0.0	-12.6	19.4	-0.0
LL	6.8	10.4	-0.0	-6.8	10.4	-0.0
RBDQEW	-0.0	0.8	-0.0	0.0	0.8	-0.0
EQ1	-0.5	-0.2	-0.0	0.5	0.2	-0.0
RBUPEO	-0.0	-0.8	-1.0	-0.0	-0.8	-1.0
WL1	-13.2	-18.5	-0.0	5.6	-13.5	-0.0
WL2	-10.4	-10.6	-0.0	2.8	-5.6	-0.0
WL3	-5.6	-13.5	-0.0	13.2	-17.5	-0.0
WL4	-2.8	-5.6	-0.0	6.8	-10.4	-0.0
LBWL1	-5.4	-17.2	-0.0	6.6	-14.2	-0.0
RBLPWL	0.0	-2.4	-3.4	-0.0	-2.4	-3.4
LWL2	-6.6	-14.2	-0.0	5.4	-17.2	-0.0
WL3	-2.6	-9.2	-0.0	1.8	-6.2	-0.0
LWL4	-1.8	-6.2	-0.0	0.6	-3.2	-0.0
SBAL	12.6	19.4	-0.0	-12.6	19.4	-0.0
RS	10.8	11.2	-0.0	-10.8	19.2	-0.0

LOAD GROUP DESCRIPTION

DL	Roof Dead Load
CDLL	Roof Dead Load (parallel to plane of frame)
SNOW	Roof Snow Load
LL	Roof Live Load
RBWEG	Downward Acting Roof Brace Load from Long. Seismic
LBWEG	Lateral Seismic Load (parallel to plane of frame)
RBUEP	Upward Acting Roof Brace Load from Long. Seismic
WL1	Wind from Left to Right with +GCP
WL2	Wind from Left to Right with -GCP
WR1	Wind from Right to Left with +GCP
WR2	Wind from Right to Left with -GCP
LWL1	Windward Corner Load with +GCP
WRUL1	Upward Acting Roof Load from Long. Wind
LWL2	Windward Corner Load with -GCP
LWL3	Windward Corner Load with -GCP
LR1	Windward Corner Right with -GCP
LR2	Code Balanced Load from Roof Snow Load
RS	Unbalanced Roof Roof Snow Load

FRAME ID #	2	USER NAME:	vmusac03pdr15	DATE:	11/24/25	PAGE:	2 - 2
CS 70.18/25,	2010.11/2.	JOB NAME:		FILE:	frame_3tra		
[S] ALL reactions for EACH LOAD GROUP							
*LOCATION: Gridlines							
NOTES [1] All reactions are in kips and kips-ft.							
[2] The seismic overstrength factor of 1.5 is not included in the "RBDWEQ" and "RBDPUQ" Load group reactions.							
[3] Seismic "BASE-ONLY" combination reactions include an overstrength factor of: 2.500							
[4] Primary wind load cases are not concurrent.							
[5] X-bracing reactions (RBDPLW and RBDPUQ) are combined with LWL and LEQ groups only.							



LOAD GROUP REACTION TABLE GRIDLINES * = 3						
COLUMN	*E			*A		
LOAD GROUP	H	V	L	H	V	L
DL	1.9	3.3	-0.0	-1.9	3.3	-0.0
COLL	2.6	3.9	-0.0	-2.6	3.9	-0.0
SNOW	12.7	19.6	-0.0	-12.7	19.6	-0.0
LL	6.8	10.5	-0.0	-6.8	10.5	-0.0
RBDQEW	-0.0	0.8	-0.0	0.0	0.8	-0.0
EQ	-0.0	-0.2	-0.0	0.0	-0.2	-0.0
RBUEPO	-0.0	-0.8	-1.0	-0.0	-0.8	-1.0
WL1	-13.4	-18.7	-0.0	5.6	-13.7	-0.0
WL2	-10.5	-10.7	-0.0	2.8	-5.6	-0.0
WL3	-5.6	-13.7	-0.0	13.4	-16.7	-0.0
WL4	-2.8	-5.6	-0.0	10.5	-10.7	-0.0
LWL1	-5.5	-17.4	-0.0	6.7	-14.4	-0.0
RBUPFL	0.0	-2.4	-3.4	-0.0	-2.4	-3.4
LWL2	-6.7	-14.4	-0.0	5.5	-17.4	-0.0
LWL3	-2.7	-9.3	-0.0	3.9	-6.3	-0.0
SBAL	-3.9	-6.3	-0.0	2.7	-9.3	-0.0
SBAL	12.7	19.6	-0.0	-12.7	19.6	-0.0
RS	10.9	11.3	-0.0	-10.9	19.4	-0.0

LOAD GROUP DESCRIPTION

DL	Roof Dead Load
COLL	Roof Coll. Load
SNOW	Roof Snow Load
LL	Roof Live Load
RDWEG	Downwind Acting Roof Brace Load from Long Seismic
LDWEG	Leeward Suction Load (parallel to plane of frame)
RBUPE	Upwind Acting Roof Brace Load from Long Seismic
WL1	Wind from Left to Right with +G/Cp
WL2	Wind from Left to Right with -G/Cp
WL3	Wind from Right to Left with +G/Cp
WL4	Wind from Right to Left with -G/Cp
LWL1	Windward Corner Load with +G/Cp
LWL2	Windward Corner Load with -G/Cp
LWL3	Windward Corner Load with +G/Cp
LWL4	Windward Corner Load with -G/Cp
RS	Cyclic Suction Balanced Roof Snow Load
RS	Unbalanced Roof Snow Load

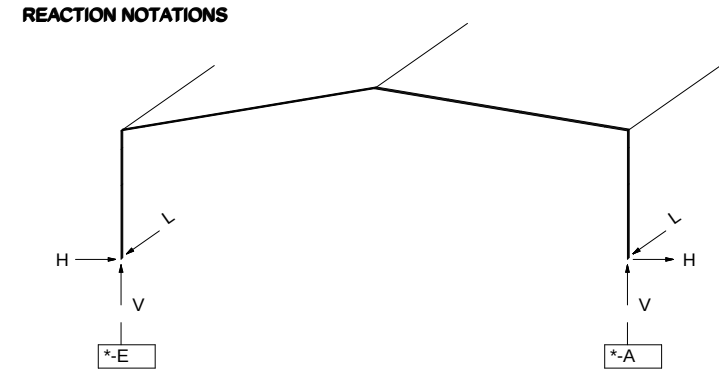
NOTES

- 1) THE REACTIONS PROVIDED ARE BASED ON THE ORDER DOCUMENTS AT THE TIME OF MAILING. ANY CHANGES TO BUILDING LOADS OR DIMENSIONS MAY CHANGE THE REACTIONS. THE REACTIONS WILL BE SUPERSEDED AND VOIDED BY ANY FUTURE MAILING.
- 2) THE REACTIONS PROVIDED HAVE BEEN CREATED WITH THE FOLLOWING LAYOUTS AND ASSUMED SERVICE:
 - a) A REACTION TABLE IS PROVIDED WITH THE REACTIONS FOR EACH LOAD GROUP.
 - b) RIGID FRAMES
 - (1) GABLED BUILDINGS
 - (a) LEFT AND RIGHT COLUMNS ARE DETERMINED AS IF VIEWING THE LEFT SIDE OF THE BUILDING, AS SHOWN ON THE ANCHOR ROD DRAWING, FROM THE OUTSIDE OF THE BUILDING.
 - (b) INTERIOR COLUMNS ARE SPACED FROM LEFT SIDE TO RIGHT SIDE.
 - (2) SINGLE SLOPE BUILDINGS
 - (a) LEFT COLUMN IS THE LOW SIDE COLUMN.
 - (b) RIGHT COLUMN IS THE HIGH SIDE COLUMN.
 - (c) INTERIOR COLUMNS ARE SPACED FROM LOW SIDE TO HIGH SIDE.
 - c) ENDWALLS
 - (1) LEFT AND RIGHT COLUMNS ARE DETERMINED AS IF VIEWING THE WALL FROM THE OUTSIDE.
 - (2) INTERIOR COLUMNS ARE SPACED FROM LEFT TO RIGHT.
 - d) ANCHOR ROD SIZE IS DETERMINED BY SHEAR AND TENSION AT THE BOTTOM OF THE BASE PLATE. THE LENGTH OF THE ANCHOR ROD AND METHOD OF LOAD TRANSFER TO THE FOUNDATION ARE TO BE DETERMINED BY THE FOUNDATION ENGINEER.
 - e) ANCHOR RODS ARE ASTM F1554 Gr. 36 MATERIAL UNLESS NOTED OTHERWISE ON THE ANCHOR ROD LAYOUT DRAWING.
 - f) X-BRACING
 - (1) ROD BRACING REACTIONS HAVE BEEN INCLUDED IN VALUES SHOWN IN THE REACTION TABLES.
 - (2) FOR IBC AND UBC BASED BUILDING CODES, WHEN X-BRACING IS PRESENT IN THE SIDEWALL, INDIVIDUAL LONGITUDINAL SEISMIC LOADS (RBUPEQ AND RBDWEQ) DO NOT INCLUDE THE AMPLIFICATION FACTOR, Q_s .
 - (3) FOR CANADA BUILDING CODE (NBC), WHEN X-BRACING IS PRESENT IN THE SIDEWALL OF ENDWALL, INDIVIDUAL LONGITUDINAL SEISMIC LOADS (RBUPEQ & RBDWEQ) ARE MULTIPLIED BY FORCE REDUCTION FACTOR, R_d , WHEN SPECIFIED SHORT-PERIOD SPECTRAL ACCELERATION RATIO $[F_s]$, $F_s \geq 0.2$ IS GREATER THAN 0.45.
 - 3) REACTIONS ARE PROVIDED AS UN-FACTORED FOR EACH LOAD GROUP APPLIED TO THE COLUMN. THE FOUNDATION ENGINEER WILL APPLY THE APPROPRIATE LOAD FACTORS AND COMBINE THE REACTIONS IN ACCORDANCE WITH THE BUILDING CODE AND DESIGN SPECIFICATIONS TO DETERMINE BEARING PRESSURES AND CONCRETE DESIGN. THE FACTORS APPLIED TO LOAD GROUPS FOR THE STEEL COLUMN DESIGN MAY BE DIFFERENT THAN THE FACTORS USED IN THE FOUNDATION DESIGN.
 - a) FOR PROJECTS USING ULTIMATE DESIGN WIND SPEEDS SUCH AS 2012 IBC, 2015 IBC, OR FLORIDA BUILDING CODE, THE WIND LOAD REACTIONS ARE AT A STRENGTH VALUE WITH A LOAD FACTOR OF 1.0.
 - b) FOR IBC CODES, THE SEISMIC REACTIONS PROVIDED ARE AT A STRENGTH LEVEL AND DO NOT CONTAIN THE RHO FACTOR.
 - c) FOR NBCC CODES, THE SEISMIC REACTIONS PROVIDED DO NOT CONTAIN THE R_p/R_s FACTOR.

THE MANUFACTURER DOES NOT PROVIDE "MAXIMUM" LOAD COMBINATION REACTIONS. HOWEVER, THE INDIVIDUAL LOAD REACTIONS PROVIDED MAY BE USED BY THE FOUNDATION ENGINEER TO DETERMINE THE APPLICABLE LOAD COMBINATIONS FOR HIS/HER DESIGN PROCEDURES AND ALLOW FOR AN ECONOMICAL FOUNDATION DESIGN.

	Revision	Date	Description	By	Ck'd
Customer: Project Name & Location: 373251					
Drawing Status:					
☐ Preliminary (Not For Construction)	☐				
☐ For Approval (Not For Construction)	☐				
☐ For Construction Permit ☐ For Erector Installation					
Scale: NOT TO SCALE					
Drawn By: Auto 11/24/25					
Checked By:					
Project Engineer:					
Job Number:					
Sheet Number: F2 of 4					
The engineer whose seal appears hereon is an employee for the manufacturer for the materials described herein. Said seal or certification is limited to the products designed and manufactured by manufacturer only.The undersigned engineer is not the overall engineer of record for this project.					

FRAME ID # 1 CS 7018/124.75 70.115/1.3	USER NAME: vmruso03pdr1s JOB NAME:	DATE: 11/24/25 FILE: frames_2_4.rfa	PAGE: 1 - 3
SUPPORT REACTIONS FOR EACH LOAD GROUP			
L'LOCATION: Gridlines: 2 4			
NOTES (1) All reactions are in kips and kip-ft. (2) The seismic overstrength factor (Omega) is not included in the "RBDWEQ" and "RBUPEQ" Load Group reactions. Seismic "BASE-ONLY" combination reactions include an overstrength factor of: 2.500 (3) Primary wind load cases are not concurrent. (4) X-tracing reactions (RBPULW and RBUPEQ) are combined with LWL and LELG groups only.		TIME: 20:29:55	



COLUMN	*E			*A		
LOAD GROUP	H	V	L	H	V	L
LS	10.8	19.2	-0.0	-10.8	11.2	-0.0
RBDWLW	-0.0	2.4	-0.0	0.0	2.4	-0.0

LOAD GROUP DESCRIPTION		
LS	:	Unbalanced Left Roof Snow Load
RBDWLW	:	Downward Acting Rod Brace Load from Long Wind

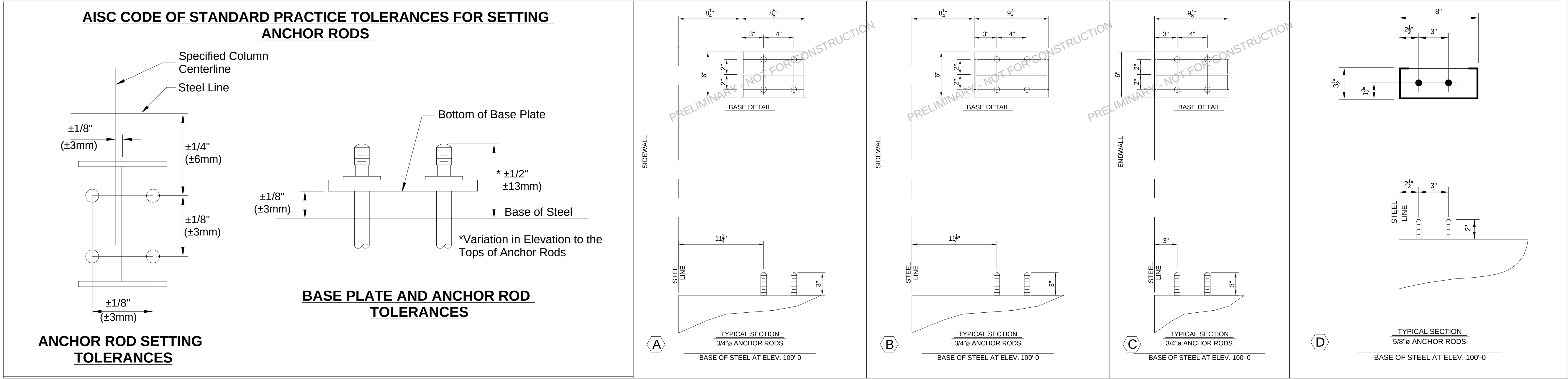
NOTES

- 1) THE REACTIONS PROVIDED ARE BASED ON THE ORDER DOCUMENTS AT THE TIME OF MAILING. ANY CHANGES TO BUILDING LOADS OR DIMENSIONS MAY CHANGE THE REACTIONS. THE REACTIONS WILL BE SUPERSEDED AND VOIDED BY ANY FUTURE MAILING.
- 2) THE REACTIONS PROVIDED HAVE BEEN CREATED WITH THE FOLLOWING LAYOUT, UNLESS NOTED OTHERWISE.
 - a) A REACTION TABLE IS PROVIDED WITH THE REACTIONS FOR EACH LOAD GROUP.
 - b) RIGID FRAMES
 - (1) GABLED BUILDINGS
 - (a) LEFT AND RIGHT COLUMNS ARE DETERMINED AS IF VIEWING THE LEFT SIDE OF THE BUILDING, AS SHOWN ON THE ANCHOR ROD DRAWING, FROM THE OUTSIDE OF THE BUILDING.
 - (b) INTERIOR COLUMNS ARE SPACED FROM LEFT SIDE TO RIGHT SIDE.
 - (2) SINGLE SLOPE BUILDINGS
 - (a) LEFT COLUMN IS THE LOW SIDE COLUMN.
 - (b) RIGHT COLUMN IS THE HIGH SIDE COLUMN.
 - (c) INTERIOR COLUMNS ARE SPACED FROM LOW SIDE TO HIGH SIDE.
 - c) ENDWALLS
 - (1) LEFT AND RIGHT COLUMNS ARE DETERMINED AS IF VIEWING THE WALL FROM THE OUTSIDE.
 - (2) INTERIOR COLUMNS ARE SPACED FROM LEFT TO RIGHT.
 - d) ANCHOR ROD SIZE IS DETERMINED BY SHEAR AND TENSION AT THE BOTTOM OF THE BASE PLATE. THE LENGTH OF THE ANCHOR ROD AND METHOD OF LOAD TRANSFER TO THE FOUNDATION ARE TO BE DETERMINED BY THE FOUNDATION ENGINEER.
 - e) ANCHOR RODS ARE ASTM F1554 Gr. 36 MATERIAL UNLESS NOTED OTHERWISE ON THE ANCHOR ROD LAYOUT DRAWING.
 - f) X-BRACING
 - (1) ROD BRACING REACTIONS HAVE BEEN INCLUDED IN VALUES SHOWN IN THE REACTION TABLES.
 - (2) FOR IBC AND UBC BASED BUILDING CODES, WHEN X-BRACING IS PRESENT IN THE SIDEWALL, INDIVIDUAL LONGITUDINAL SEISMIC LOADS (RBUPEQ AND RBDWEQ) DO NOT INCLUDE THE AMPLIFICATION FACTOR, Q_h .
 - (3) FOR CANADA BUILDING CODE (NBC), WHEN X-BRACING IS PRESENT IN THE SIDEWALL OR ENDWALL, INDIVIDUAL LONGITUDINAL SEISMIC LOADS (RBUPEQ & RBDWEQ) ARE MULTIPLIED BY FORCE REDUCTION FACTOR, R_d , WHEN SPECIFIED SHORT-PERIOD SPECTRAL ACCELERATION RATIO $\mu_{F,S}$ (0.2) IS GREATER THAN 0.45.
 - 3) REACTIONS ARE PROVIDED AS UN-FACTORED FOR EACH LOAD GROUP APPLIED TO THE COLUMN. THE FOUNDATION ENGINEER WILL APPLY THE APPROPRIATE LOAD FACTORS AND COMBINE THE REACTIONS IN ACCORDANCE WITH THE BUILDING CODE AND DESIGN SPECIFICATIONS TO DETERMINE BEARING PRESSURES AND CONCRETE DESIGN. THE FACTORS APPLIED TO LOAD GROUPS FOR THE STEEL COLUMN DESIGN MAY BE DIFFERENT THAN THE FACTORS USED IN THE FOUNDATION DESIGN.
 - a) FOR PROJECTS USING ULTIMATE DESIGN WIND SPEEDS SUCH AS 2012 IBC, 2015 IBC, OR FLORIDA BUILDING CODE, THE WIND LOAD REACTIONS ARE AT A STRENGTH VALUE WITH A LOAD FACTOR OF 1.0.
 - b) FOR IBC CODES, THE SEISMIC REACTIONS PROVIDED ARE AT A STRENGTH LEVEL AND DO NOT CONTAIN THE RHO FACTOR.
 - c) FOR NBCC CODES, THE SEISMIC REACTIONS PROVIDED DO NOT CONTAIN THE $R_F R_o$ FACTOR.

THE MANUFACTURER DOES NOT PROVIDE "MAXIMUM" LOAD COMBINATION REACTIONS. HOWEVER, THE INDIVIDUAL LOAD REACTIONS PROVIDED MAY BE USED BY THE FOUNDATION ENGINEER TO DETERMINE THE APPLICABLE LOAD COMBINATIONS FOR HIS/HER DESIGN PROCEDURES AND ALLOW FOR AN ECONOMICAL FOUNDATION DESIGN.

[illegible]

NOT FOR CONSTRUCTION

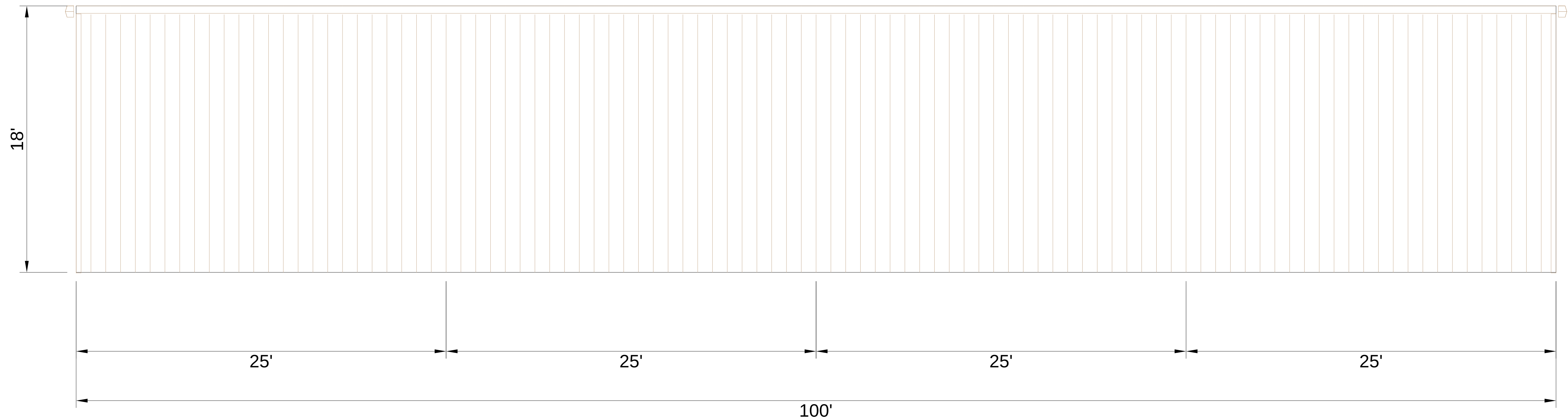


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ANCHOR ROD SECTION DETAILS - (A) 70 x 100 - 18

Customer: Project Name & Location: 373251		Revision	Date	Description	By	Ckd
☐ Preliminary (Not For Construction) ☐ For Approval (Not For Construction)						
☐ For Construction Permit ☐ For Erector Installation						
Scale: NOT TO SCALE						
Drawn By: Auto 11/24/25						
Checked By:						
Project Engineer:						
Job Number:						
Sheet Number: F4 of 4						
The engineer whose seal appears hereon is an employee for the manufacturer for the materials described herein. Said seal or certification is limited to the products designed and manufactured by manufacturer only.The undersigned engineer is not the overall engineer of record for this project.						

NOT FOR CONSTRUCTION

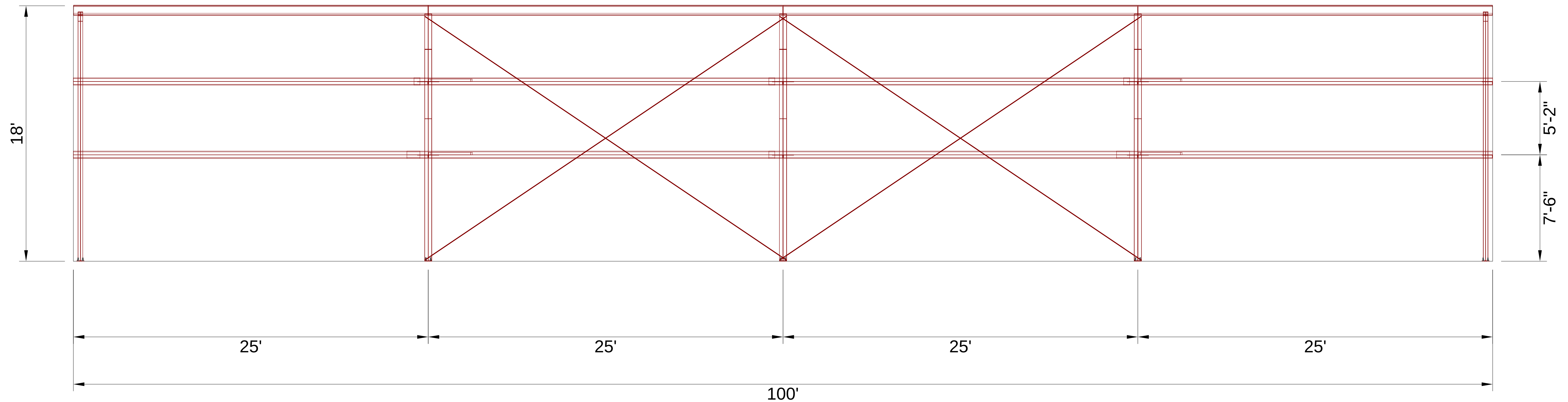


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BACK ARCHITECTURAL - (A) 70 x 100 - 18

<i>Builder:</i> FORO STEEL BUILDINGS USA <i>Customer:</i>		<i>CONTACT:</i> <i>COUNTY:</i> <i>Drawing Status:</i> ☐ Preliminary ☐ For Approval	
Scale: NOT TO SCALE			
VERSION ABS 2.1.9		PAPER SIZE 22x34	
ESTIMATOR		DATE 11/24/025	
JOB NAME 373251			
MEMBER			
			
The engineer whose seal appears hereon is an employee for the manufacturer for the materials described herein. Said seal or certification is limited to the products designed and manufactured by manufacturer only. The undersigned engineer is not the overall engineer of record for this project.			

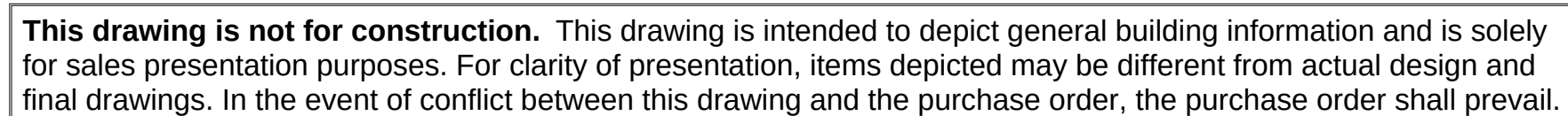
NOT FOR CONSTRUCTION



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BACK STRUCTURAL ELEVATION - (A) 70 x 100 - 18

Builder: TORO STEEL BUILDINGS USA		CONTACT: COUNTY:		Customer:	
Drawing Status:		☐ Preliminary		☐ For Construction Permit	
		☐ For Approval		☐ For Erector Installation	
Scale: NOT TO SCALE					
VERSION ABS 2.1.9		PAPER SIZE 22x34			
ESTIMATOR		DATE 11/24/025			
JOB NAME 373251					
MEMBER 					
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MANUFACTURER RESERVES THE RIGHT TO CHANGE THE FINAL DESIGN. IF DESIGN INFORMATION (IE. CLEARANCES, BASE PLATE/ANCHOR ROD DESIGN) IS TO BE USED FOR CONSTRUCTION MANUFACTURER MUST BE NOTIFIED PRIOR TO ACCEPTANCE OF ORDER.

IT IS THE BUILDERS RESPONSIBILITY TO COMMUNICATE TO MANUFACTURER THE NEED TO HOLD TO ANY PRELIMINARY DESIGN INFORMATION PROVIDED BY MANUFACTURER! MANUFACTURER WILL NOT BE LIABLE FOR ANY CHANGES IN FINAL DESIGN IF THE BUILDER DOES NOT COMMUNICATE TO MANUFACTURER!

CROSS SECTION AT FRAME LINES "2-4" - (A) 70 x 100 - 18

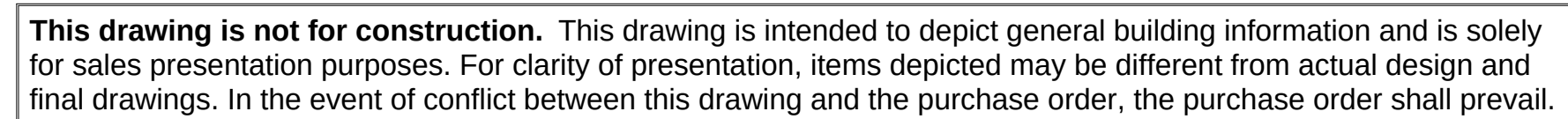
Builder: TORO STEEL BUILDINGS USA		Customer:
Drawing Status: ☐ Preliminary ☐ For Approval		☐ For Construction Permit ☐ For Erector Installation
Scale: NOT TO SCALE		
VERSION	PAPER SIZE	
ABS 2.1.9	22x34	
ESTIMATOR	DATE	
	11/24/025	
JOB NAME		
373251		
MEMBER		
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FRONT ARCHITECTURAL - (A) 70 x 100 - 18

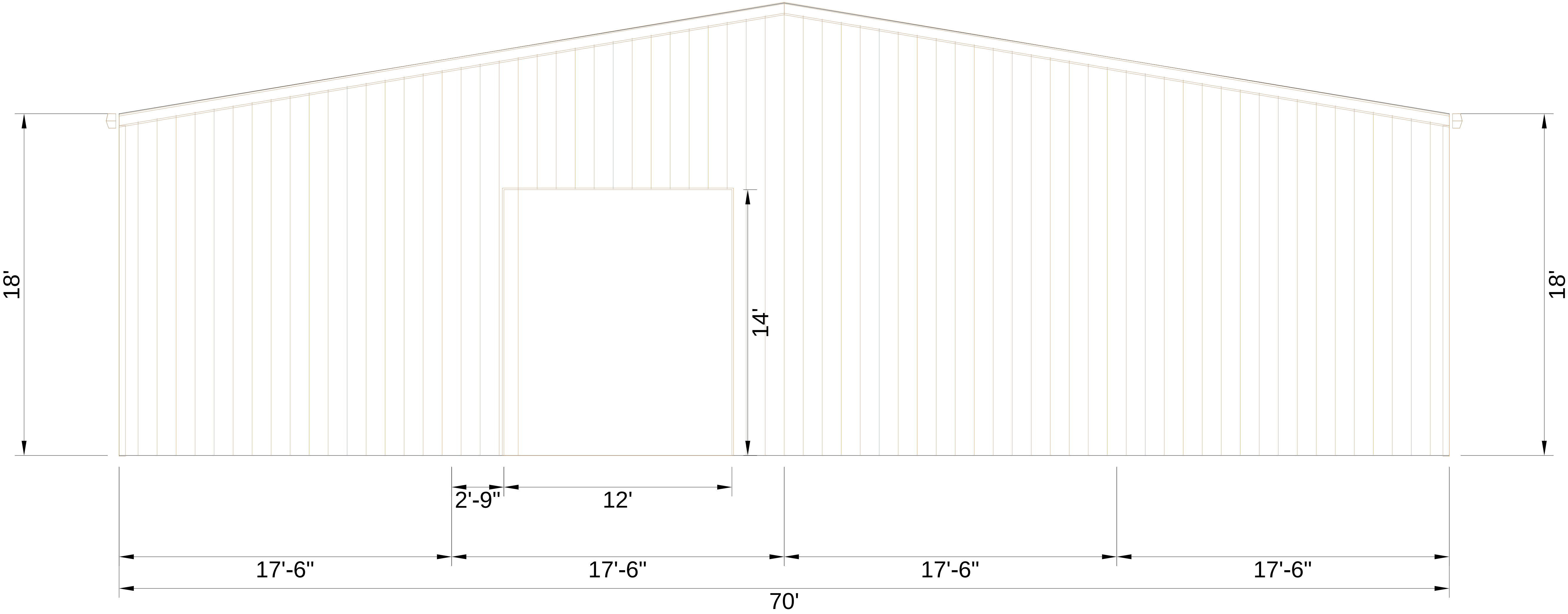
Builder: TORO STEEL BUILDINGS USA		Customer:			
CONTACT: COUNTY:					
Drawing Status:		Preliminary ☐	For Construction Permit ☐	Preliminary ☐	For Erector Installation ☐
Scale: NOT TO SCALE					
VERSION ABS 2.1.9		PAPER SIZE 22x34			
ESTIMATOR		DATE 11/24/025			
JOB NAME 373251					
MEMBER  The engineer whose seal appears hereon is an employee for the manufacturer for the materials described herein. Said seal or certification is limited to the products designed and manufactured by manufacturer only. The undersigned engineer is not the overall engineer of record for this project.					



FRONT STRUCTURAL ELEVATION - (A) 70 x 100 - 18

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NOT FOR CONSTRUCTION

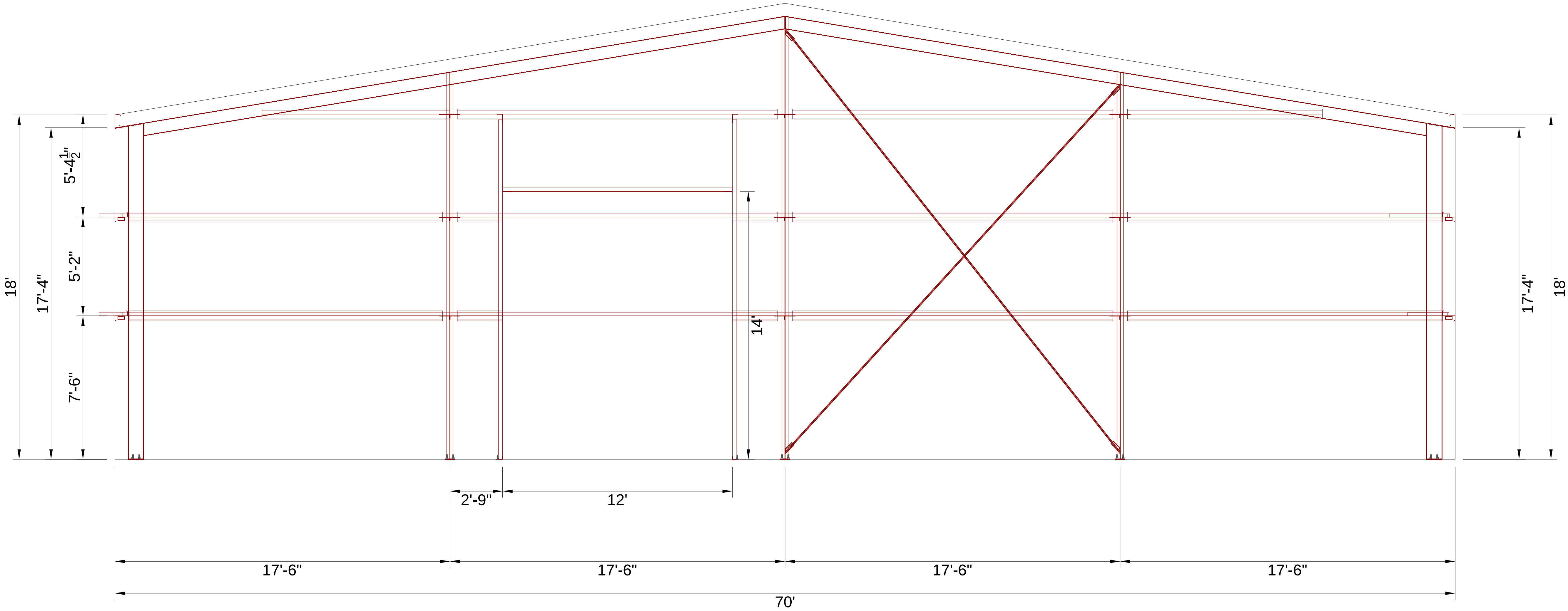


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LEFT ARCHITECTURAL - (A) 70 x 100 - 18

Customer:		CONTACT:		COUNTY:	
Builder:		TORO STEEL BUILDINGS USA		Drawing Status:	
☐ Preliminary		☐ For Construction Permit		☐ For Erector Installation	
ESTIMATOR		DATE		11/24/025	
JOB NAME		373251			
MEMBER		MBMA			
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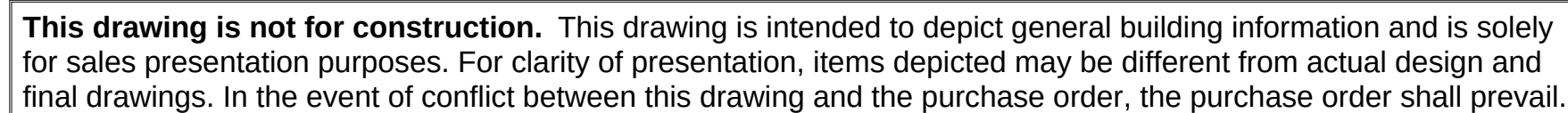
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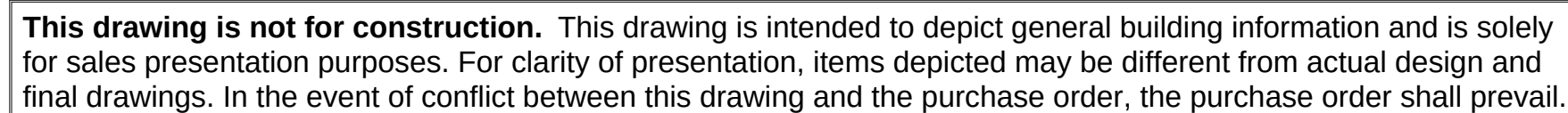
LEFT STRUCTURAL ELEVATION - (A) 70 x 100 - 18

		Customer:
Builder:	CONTACT:	
TORO STEEL BUILDINGS USA	COUNTY:	
Drawing Status:		☐ Preliminary
		☐ For Approval
		☐ For Construction Permit
		☐ For Erector Installation
Scale: NOT TO SCALE		
VERSION	PAPER SIZE	
ABS 2.1.9	22x34	
ESTIMATOR	DATE	
373251	11/24/025	
JOB NAME		
373251		
MEMBER		
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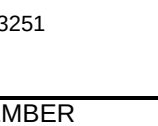


RIGHT ARCHITECTURAL - (A) 70 x 100 - 18

Builder: TORO STEEL BUILDINGS USA	Customer:	Drawing Status: ☐ Preliminary ☐ For Construction Permit ☐ For Approval ☐ For Erector Installation
Scale: NOT TO SCALE		
VERSION ABS 2.1.9	PAPER SIZE 22x34	
ESTIMATOR	DATE 11/24/025	
JOB NAME 373251		
MEMBER 		
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RIGHT STRUCTURAL ELEVATION - (A) 70 x 100 - 18

Builder: TORO STEEL BUILDINGS USA Customer:		For Construction Permit For Erector Installation Preliminary For Approval	
Scale: NOT TO SCALE			
VERSION ABS 2.1.9		PAPER SIZE 22x34	
ESTIMATOR		DATE 11/24/025	
JOB NAME 373251			
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